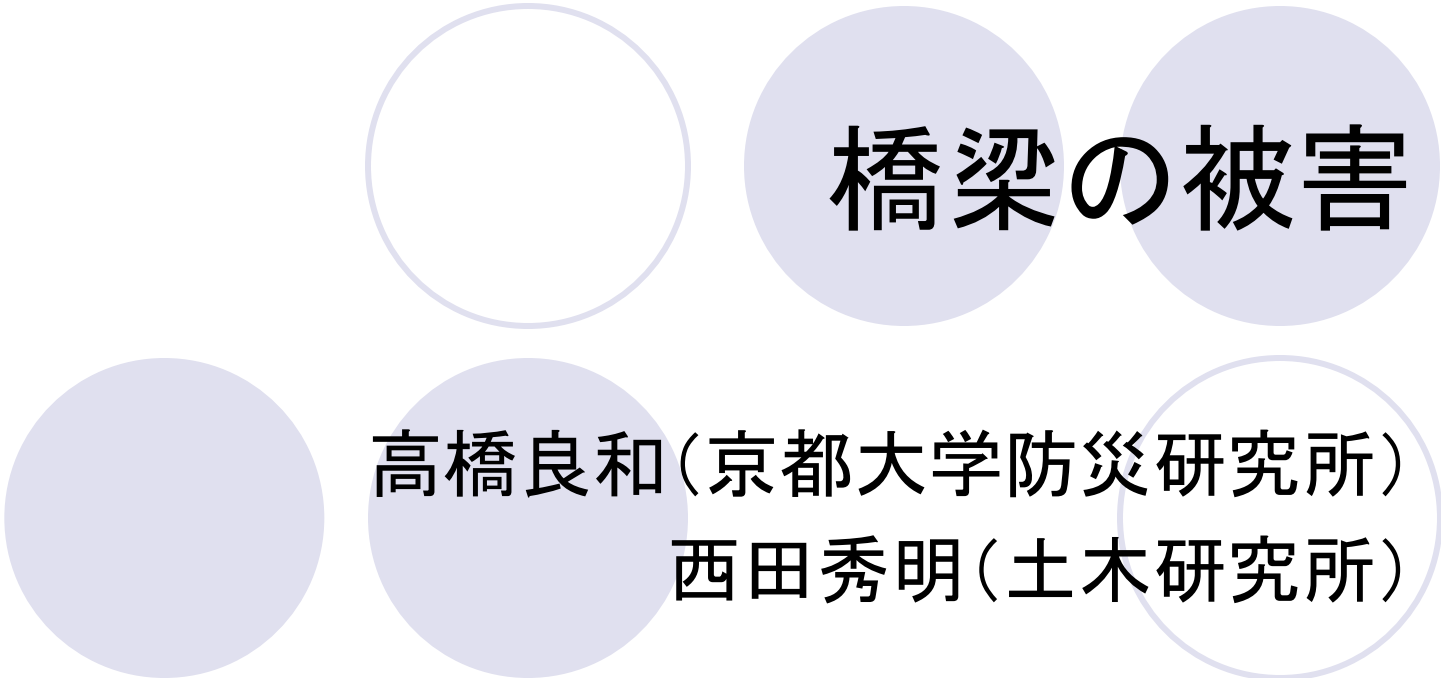


2011年2月22日ニュージーランド地震被害
土木学会・地盤工学会合同報告会(2011年3月11日)

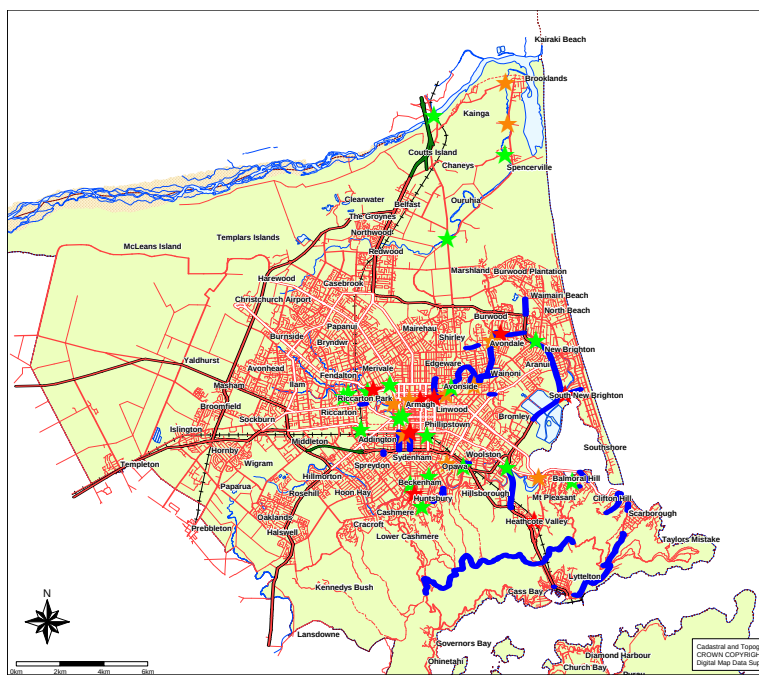


橋梁の被害

高橋良和(京都大学防災研究所)
西田秀明(土木研究所)

橋梁調査位置

- 調査前資料(2/27)では,
- 9橋が不通
- 10橋が通行制限



Road Closures – Updated 7:00pm, Sunday 27th Feb...

<http://canterburyearthquake.org.nz/road-closures/>

Bridges

Bridge	Status	Notes
Armagh Street (to Hagley Park)	Restricted access	Limited access
Avondale/New Brighton	Closed	
Blenheim Rd	Open	
Bowenvale Ave	Open	
Bridge St	Closed	
Cambridge/Durham St	Open	
Colombo St (Cambridge Tce over Avon River)	Restricted access	Limited access
Colombo St (Ernlea Tce over Heathcote river)	Open	
Colombo St rail overbridge	Closed	
Durham St overbridge	Restricted access	One lane, 3500kg weight limit
Earlham Rd bridge (off Lower Styx)	Restricted access	30km/h speed limit
Ensors Rd (at Heathcote)	Restricted access	North approach cracked
Fendalton Rd (Deans Ave/Harper Ave)	Open	
Ferry Rd/Humphreys Rd (Ferrymead Bridge)	Restricted access	Light vehicles only
Fitzgerald/Avonside/Kilmore	Closed	
Gayhurst Rd/Gloucester St	Restricted access	3500kg weight limit
Harbour Road	Restricted access	30km/h speed limit

橋梁調査位置

- 調査前資料(2/28)では,
 - 9橋が不通
 - 10橋が通行制限
- クライストチャーチ周辺を含む25橋を調査(3/2~5)





No01
Chaney overpass



No02
Waimakariri brdg



No03
Kaiapoi brdg



No04
Mandeville bootbrdg



No05
Mona Vale footbrdgs



No06
Helmores Lane brdg



No07
Avonside brdg



No08
Stanmore Rd brdg



No09
Gayhurst Rd brdg



No10
an arch footbrdg



No11
Avondale Rd brdg



No12
Anzac brdg



No13
Wainoni Rd brdg



No14
South Brighton brdg



No15
Ferrymead brdg



No16
Heathcote River brdg



No17
Horotane overbrdg



No18
Port Hills Rd overpass



No19
Durham St overbrdg



No20
Moorhouse Ave overbrdg



No21
Old Waimakariri brdg



No22
Waimakariri rail brdg



No23
Belfast Rd culvert



No24
Groynes footbrdg



No25
Main South Line overbrdg

橋梁被害の概要

- 振動による被害
- 地盤変状による被害

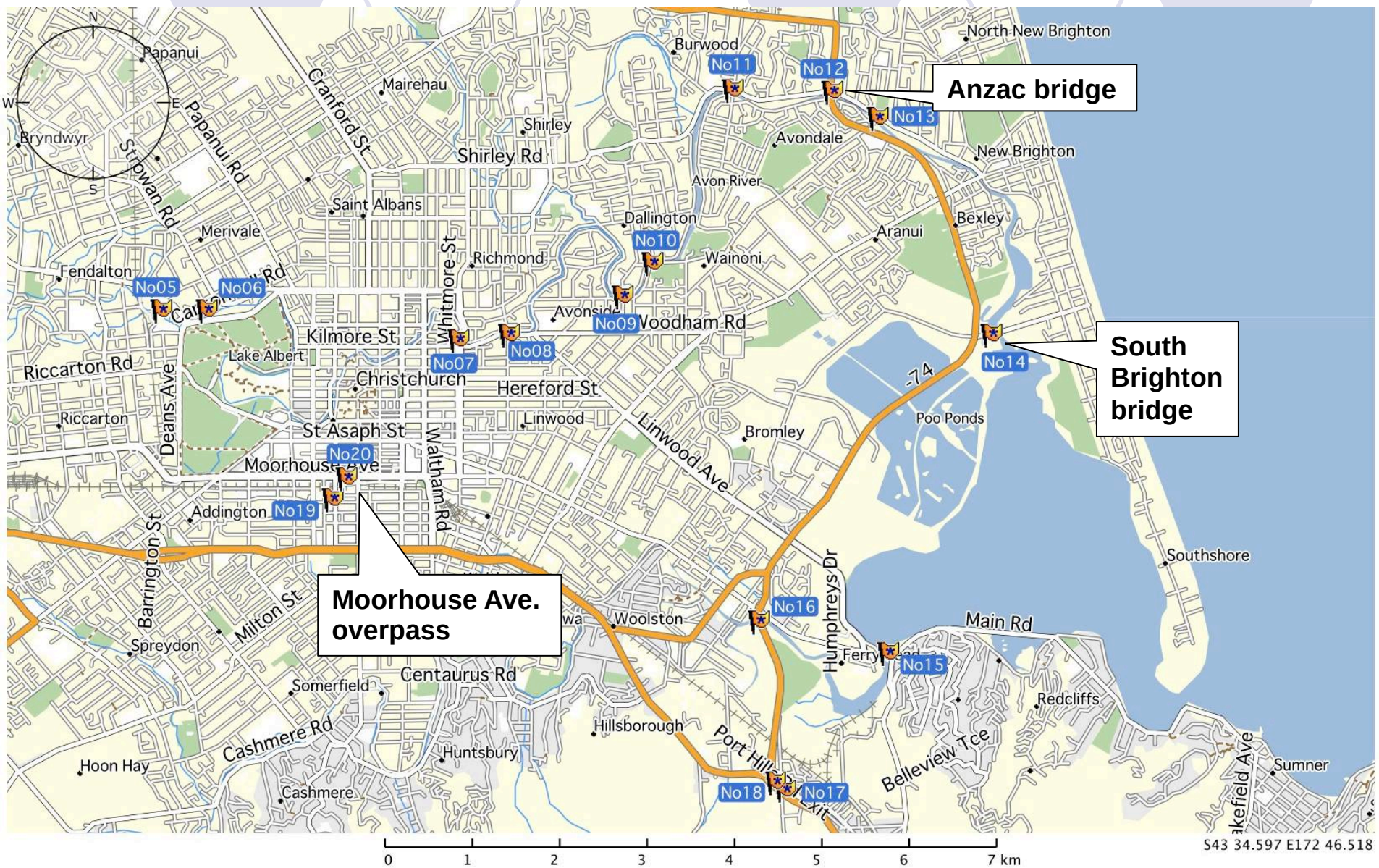




Meeting at OPUS (2011/3/4)

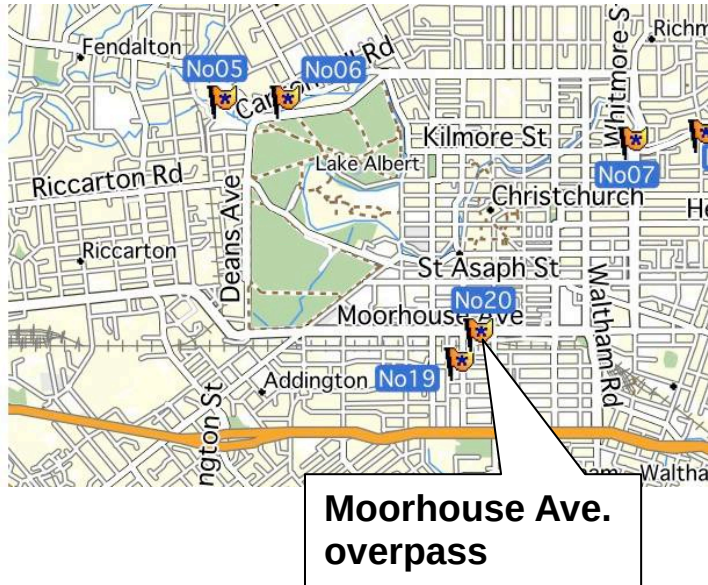


橋梁被害報告



代表的橋梁被害

Moorhouse Ave/Colombo St overpass



11径間高架橋

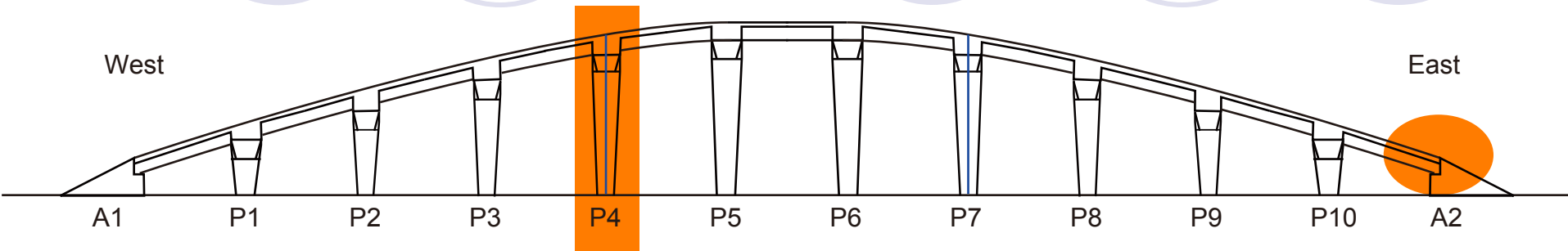
West

East

A1 P1 P2 P3 P4 P5 P6 P7 P8 P9 P10 A2

代表的橋梁被害

Moorhouse Ave/Colombo St overpass

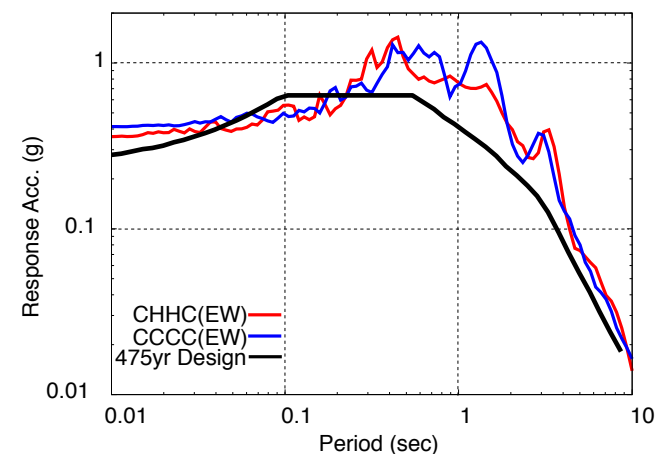
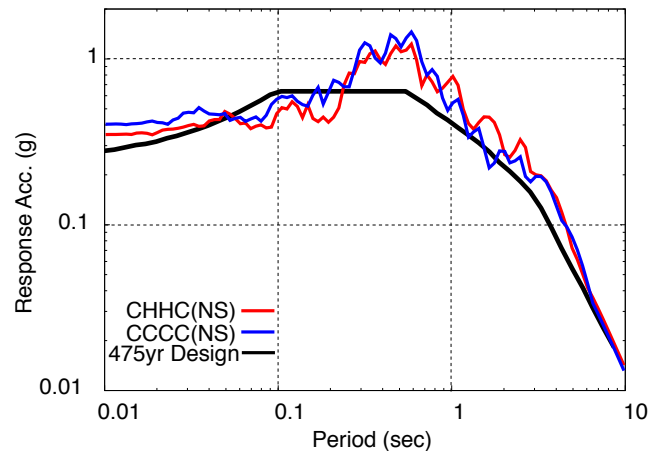
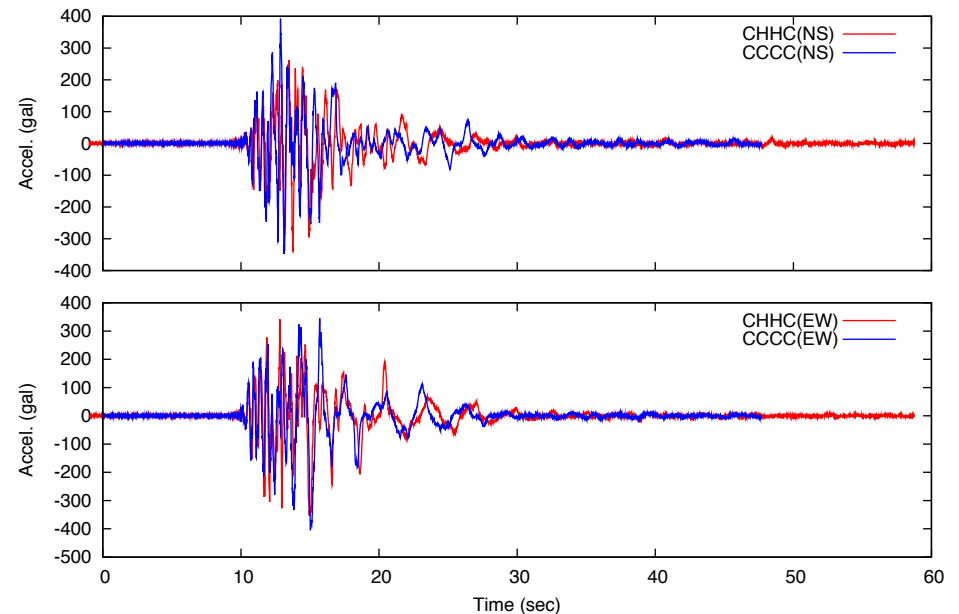


P4橋脚にせん断破壊が発生

A2橋台で桁衝突

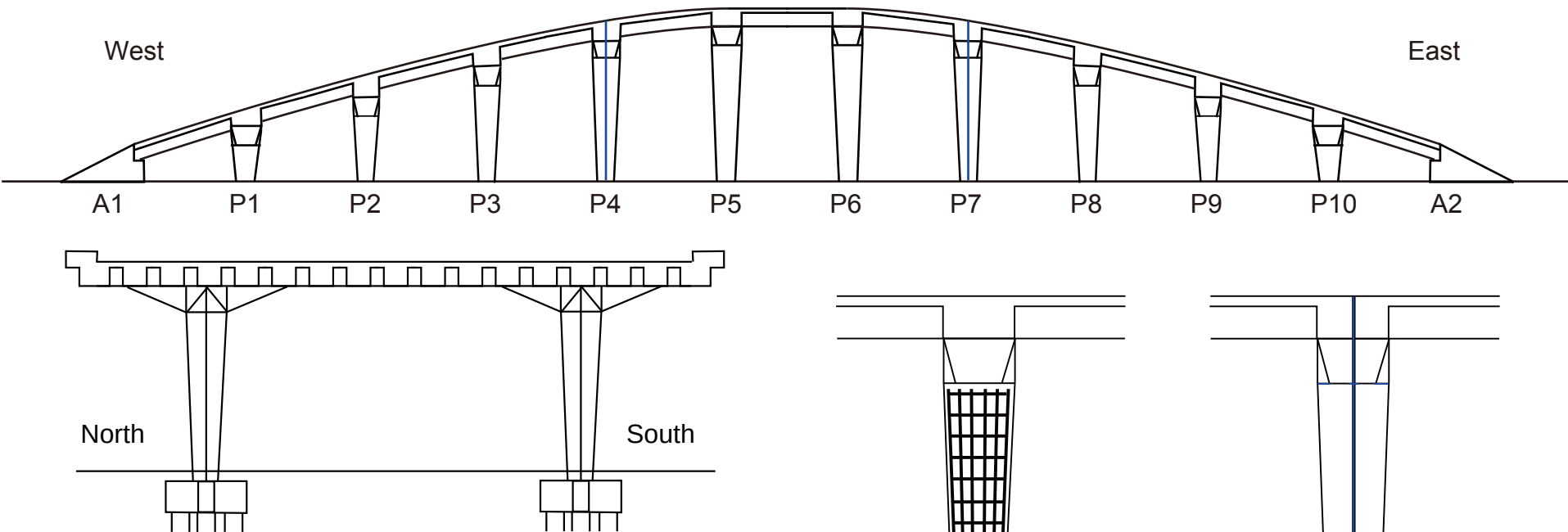


Moorhouse Ave/Colombo St overpass 周辺での地震記録



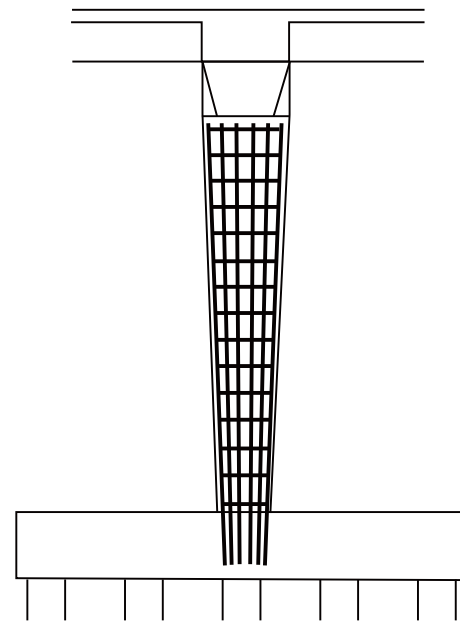
Moorhouse Ave/Colombo St overpass

構造概要

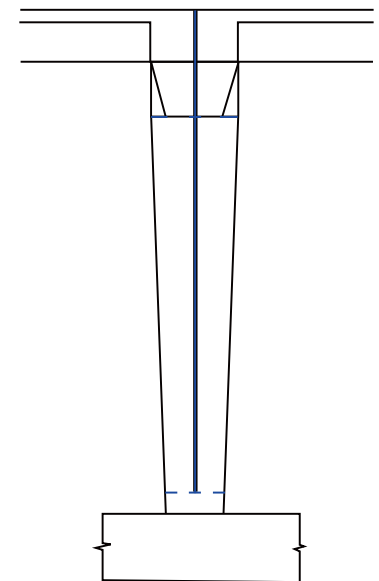


P4, P7以外は、六角形断面の
ラーメン橋脚構造であるのに対し、

P4, P7は2つの橋脚があわさって
一つの橋脚を構成している。



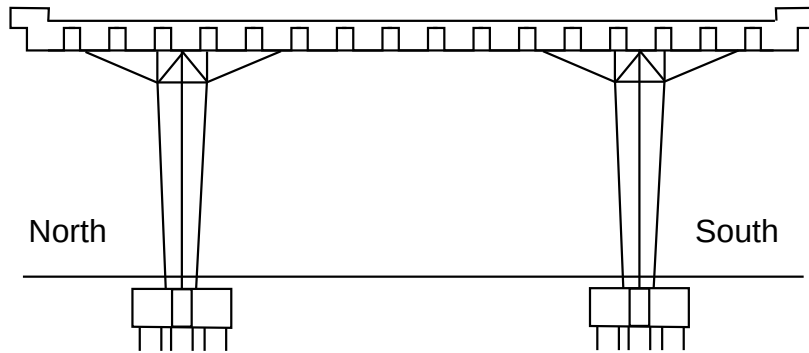
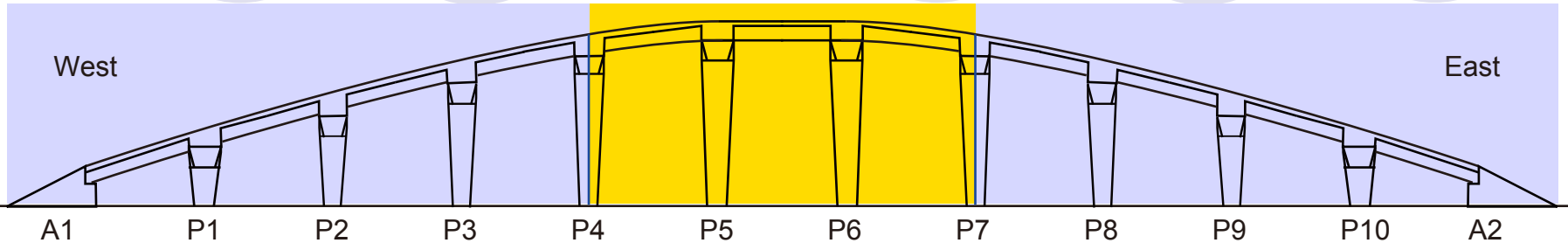
P4, P7以外



P4, P7

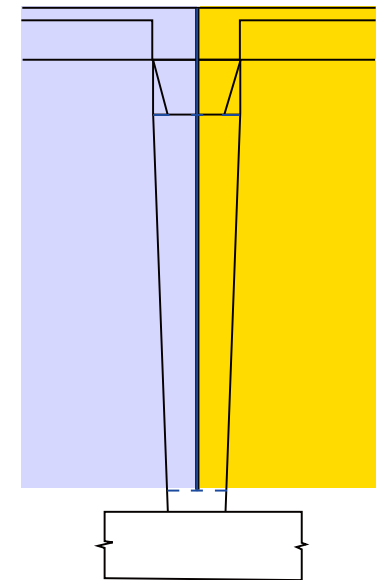
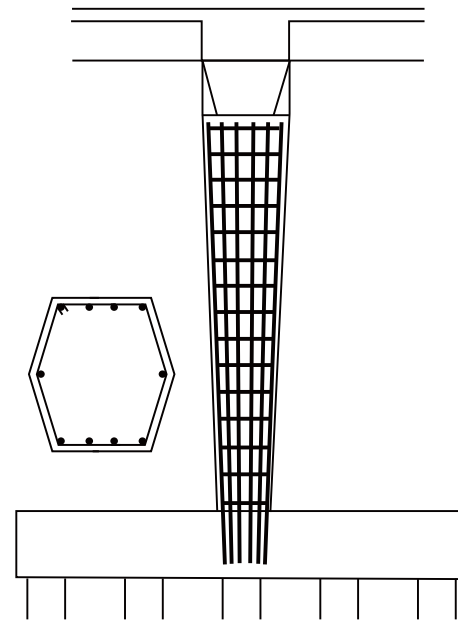
Moorhouse Ave/Colombo St overpass

構造概要



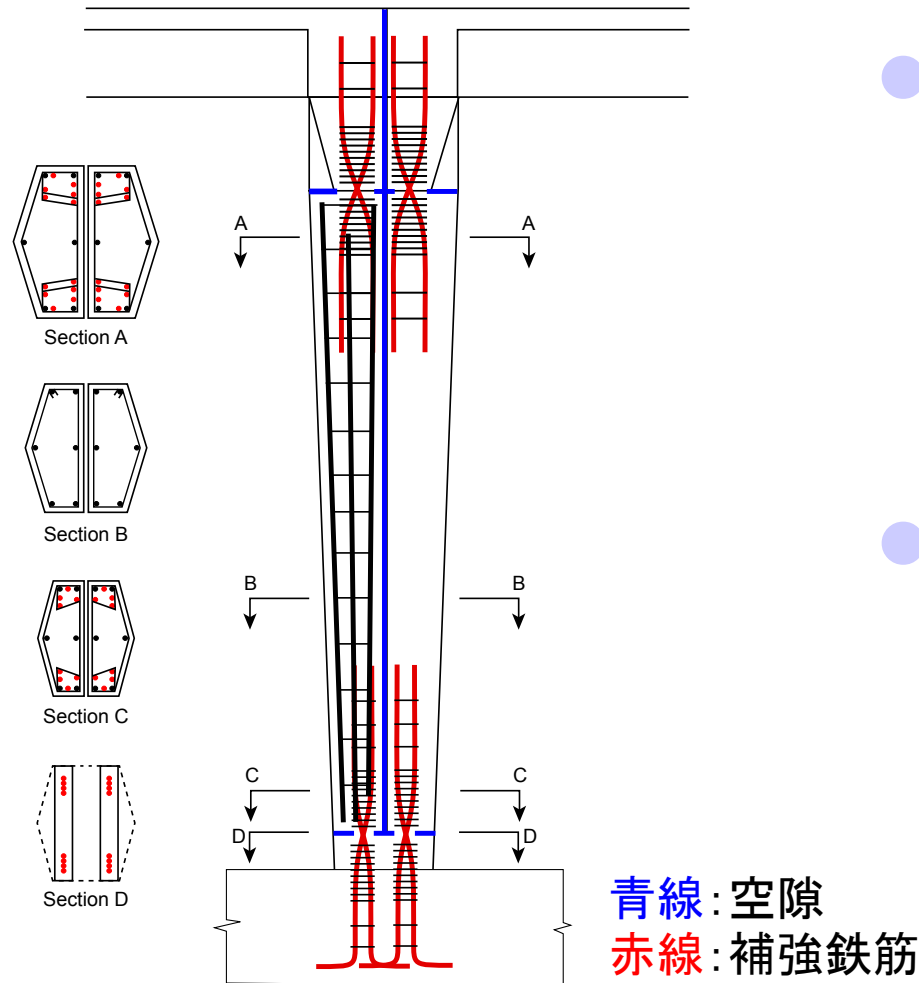
P4, P7以外は、六角形断面の
ラーメン橋脚構造であるのに対し、

P4, P7は2つの橋脚があわさって
一つの橋脚を構成している。



Moorhouse Ave/Colombo St overpass

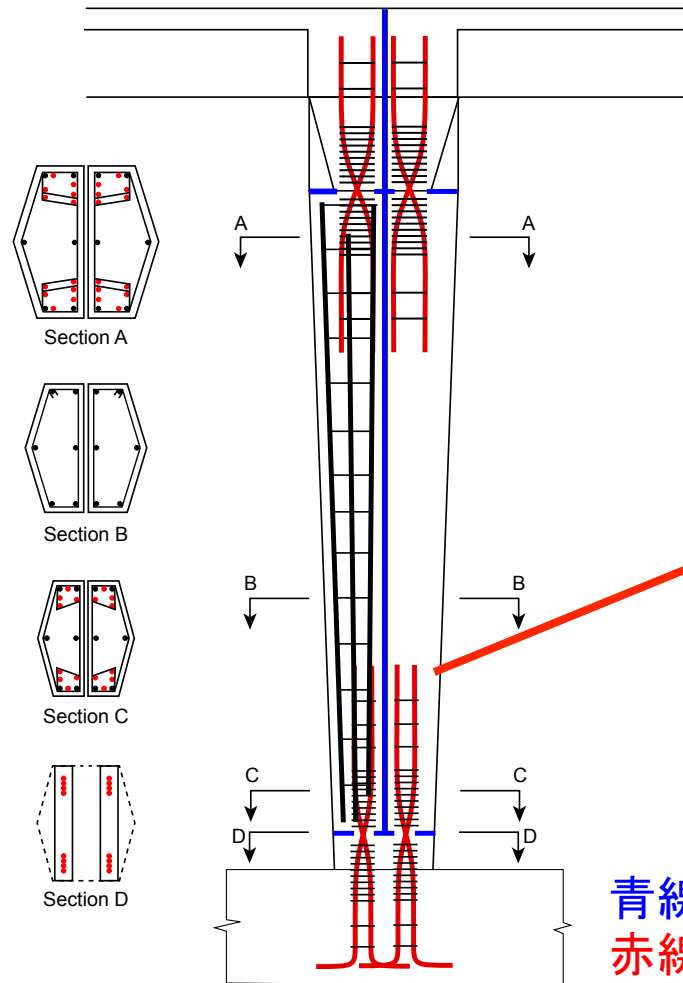
P4, P7橋脚構造詳細



- P4, P7橋脚は、2本の橋脚が重なり合っていることに加え、上下端にヒンジ構造を有している。
- ヒンジ部を形成するため、補強鉄筋を配置するとともに、断面も小さくしている。

Moorhouse Ave/Colombo St overpass

P4, P7橋脚構造詳細



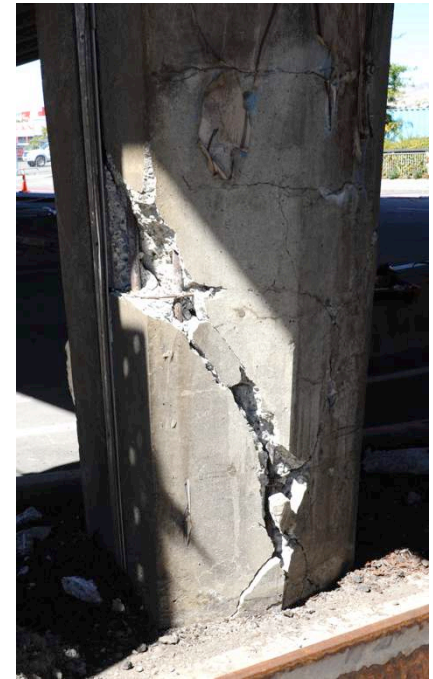
青線: 空隙
赤線: 補強鉄筋

補強鉄筋の端部が損傷部で確認

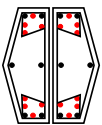
St overpass



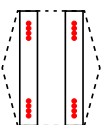
Courtesy of Prof. Kawashima



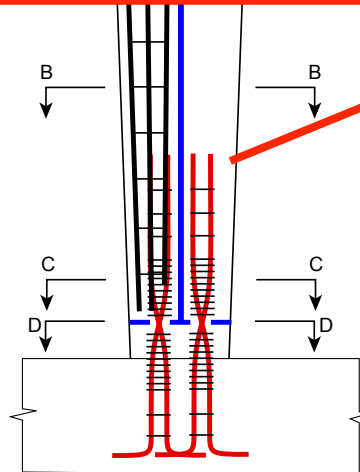
Section B



Section C



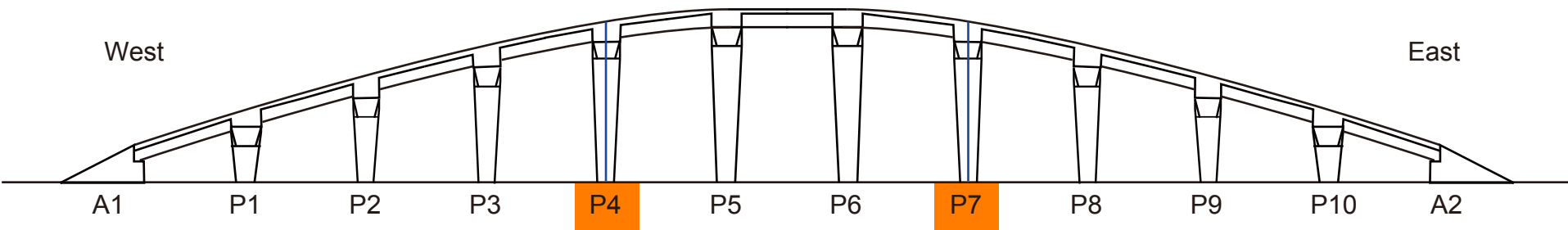
Section D



青線: 空隙
赤線: 補強鉄筋

補強鉄筋の端部が損傷部で確認

Moorhouse Ave/Colombo St overpass 構造概要



対称な橋梁だが、P4は甚大な損傷であるのに対し、P7はほぼ無損傷。

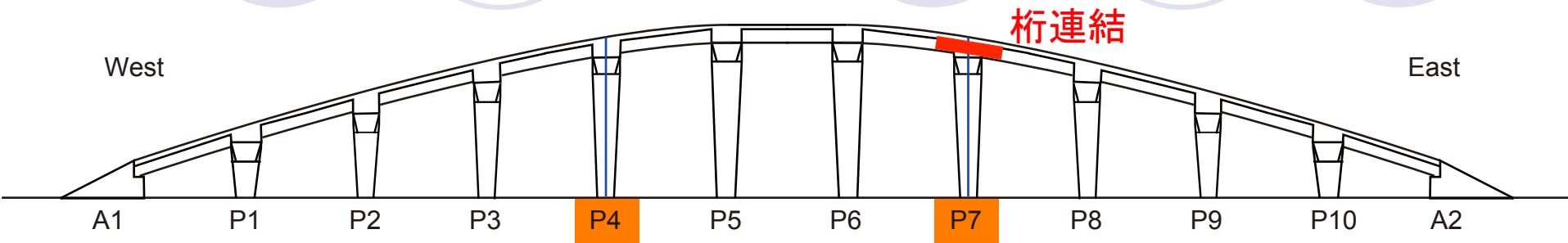


P4



P7

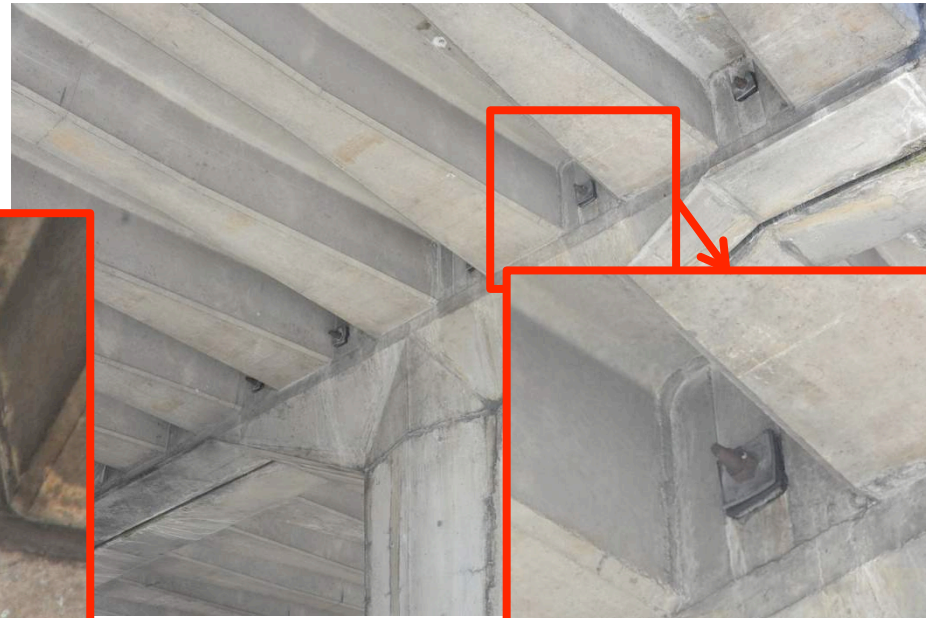
Moorhouse Ave/Colombo St overpass 構造概要



対称な橋梁だが、P4は甚大な損傷であるのに対し、P7はほぼ無損傷。



P4

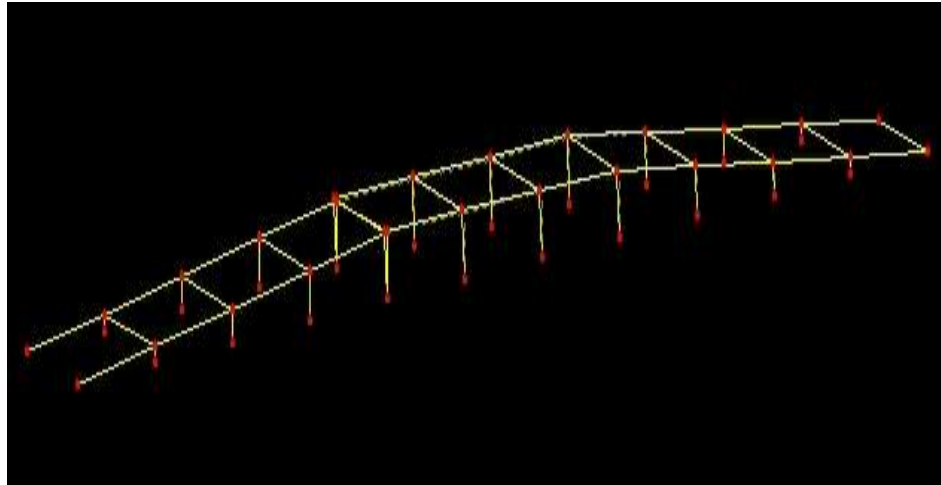


P7

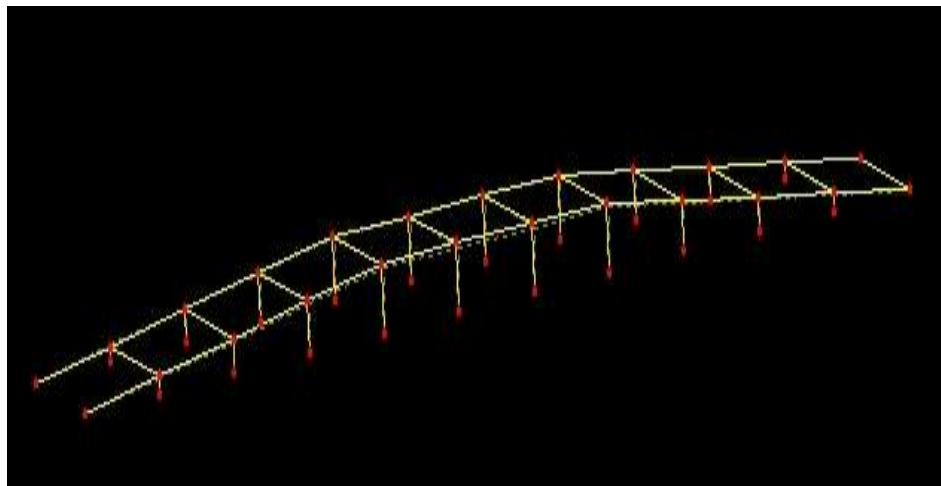
Moorhouse Ave/Colombo St overpass

桁連結有無による振動形態の差異

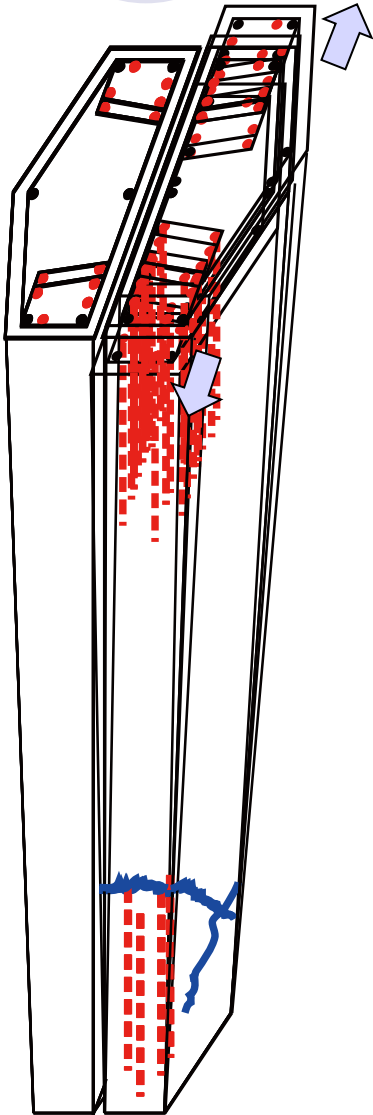
桁連結無し



桁連結有り



Moorhouse Ave/Colombo St overpass 損傷メカニズムの推定



P4西側と東側が別々に挙動し、



向北側へ応答し、
段落とし部で曲げ

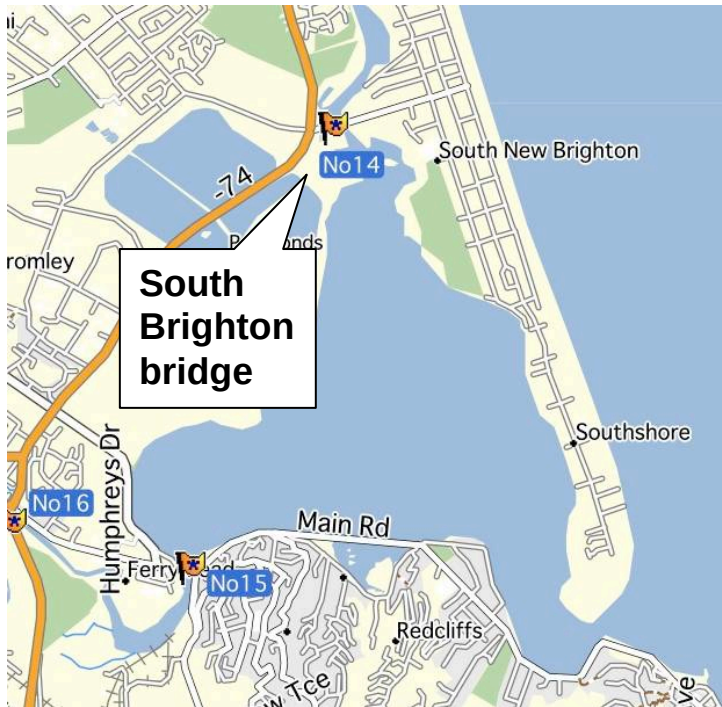
曲げせん断ひび

答し、橋脚北面の
れが発生

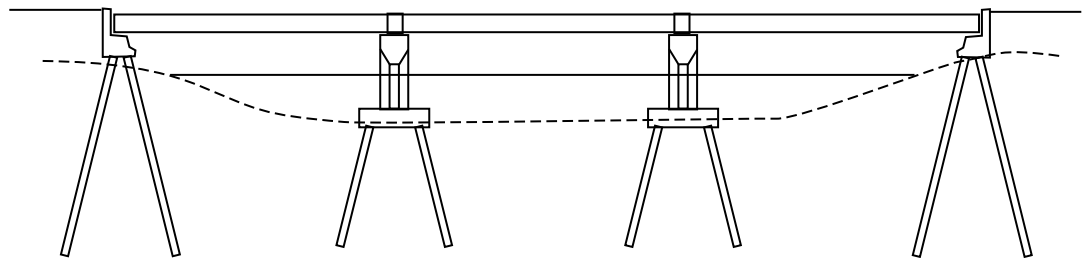
曲げせん断ひび
が交差

代表的橋梁被害

South Brighton Bridge

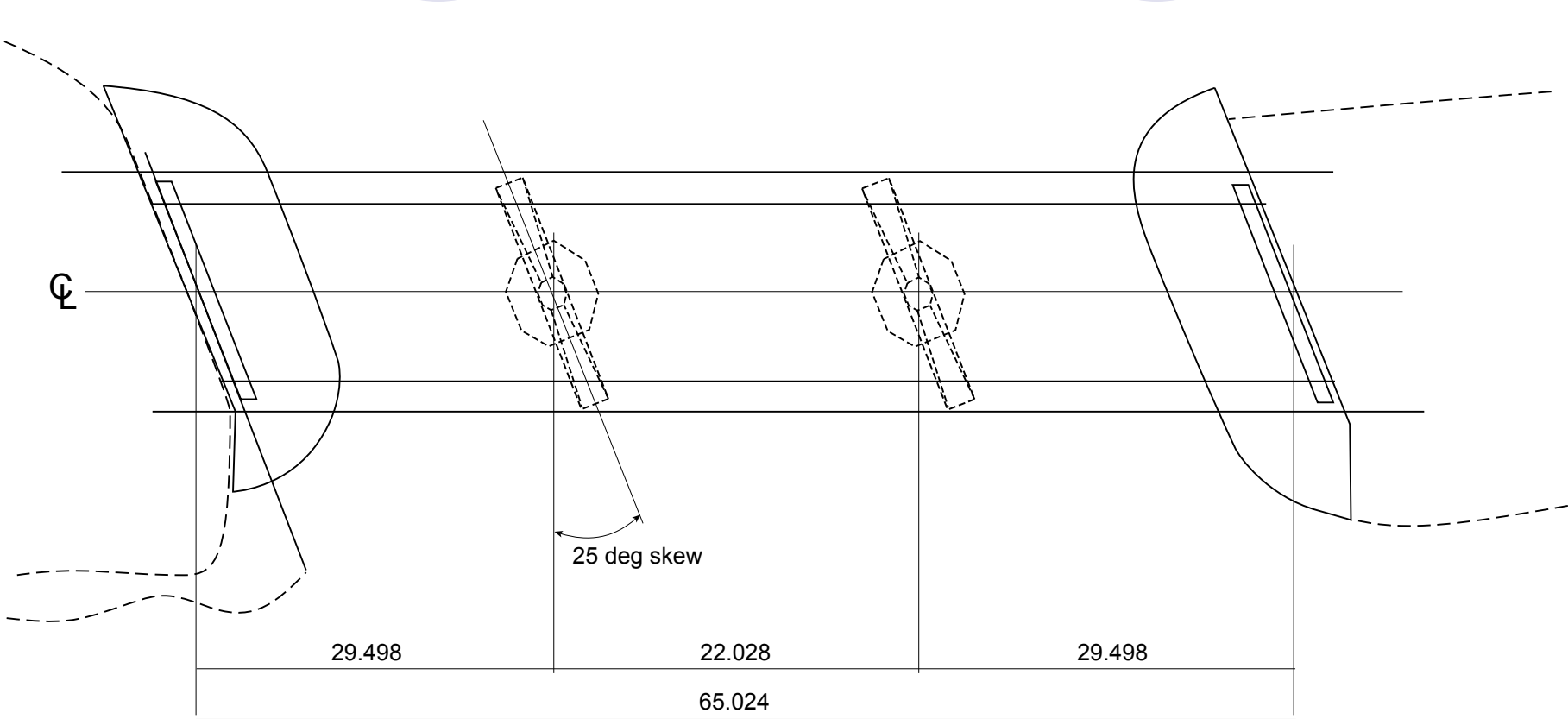
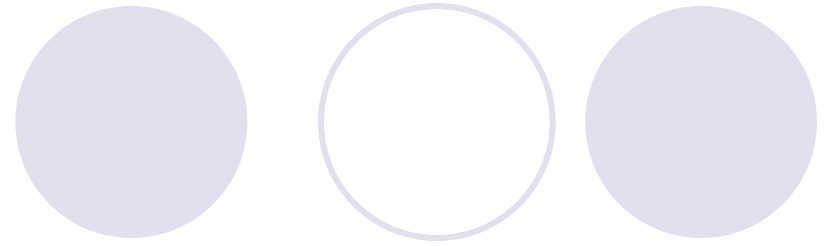


- 1980年竣工
- 二径間連続橋

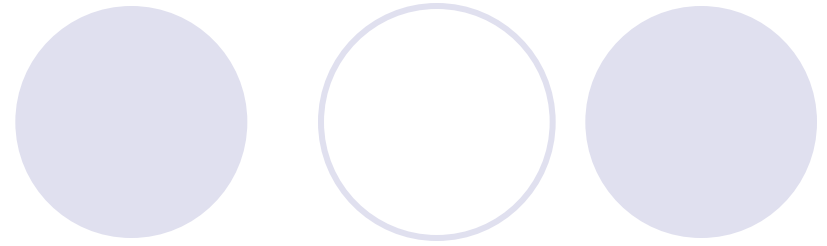


South Brighton Bridge

構造概要



South Brighton Bridge 耐震設計経緯



- Bulletin of the New Zealand National Society for Earthquake Engineering, Vol.11, No.2, 1978において、耐震設計検討が報告されている。

SEISMIC DESIGN OF SOUTH BRIGHTON BRIDGE - A DECISION AGAINST MECHANICAL ENERGY DISSIPATORS

M.J.N. Priestley* and M.J. Stockwell**

SUMMARY

The seismic design of a proposed Christchurch bridge is discussed. Initial design was based on supporting the superstructure on elastomeric bearing pads and **steel-cantilever dampers**. Dynamic analyses showed that the dampers were relatively ineffective, and that the seismic response was dictated by the characteristics of the bearing pads. Final design, based on a conventional ductile approach was found to be more economical without significant increase to seismic risk. It is shown that the decision to opt for a design incorporating mechanical energy dissipators as opposed to a monolithic pier/superstructure design will not necessarily result in a reduction in seismic response.

- 鋼製ダンパーの採用が検討され、動的解析が実施されている。

South Brighton Bridge 耐震設計経緯

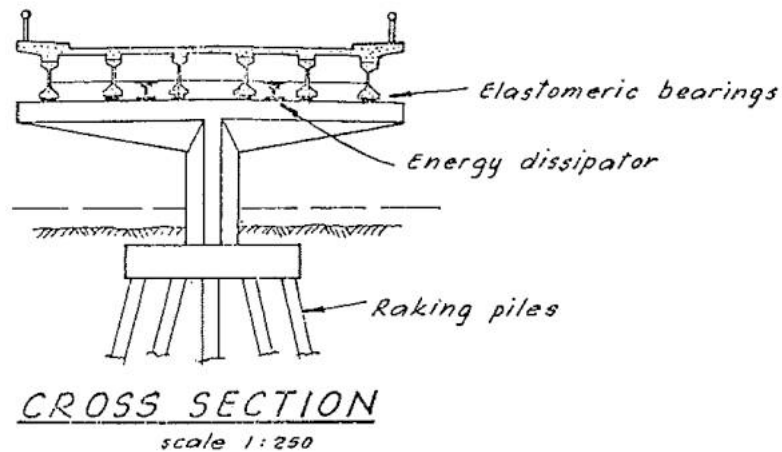


FIGURE 1: BRIDGE DETAILS

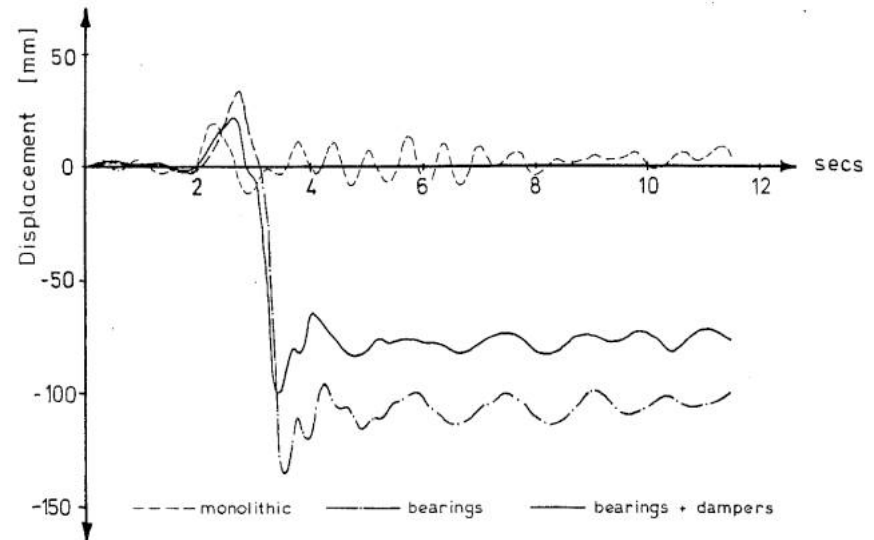
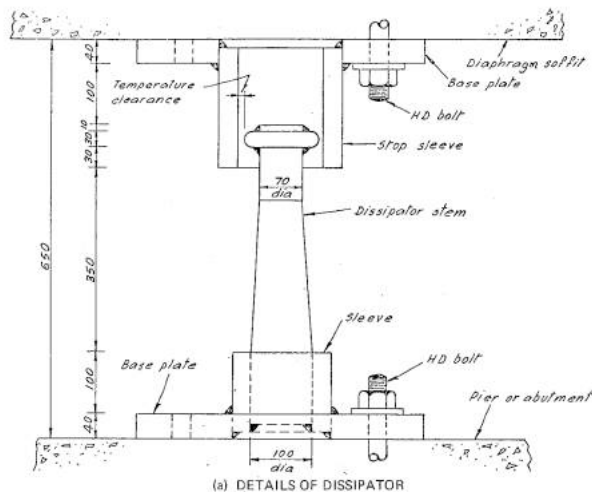


FIGURE 8: PIER DISPLACEMENT (4.73m) BUCHAREST 1977

- 積層ゴム支承＋ダンパーの採用により、El Centro波形に対して効果があったものの、卓越周期の長いBucharest波形では残留変形が大きくなり、結果的に従来型の耐震設計が採用された。



-
- Technical drawing of the bridge structure showing the deck, piers, and abutments. The drawing includes labels for 'Spread concrete embankment', 'Pier concrete', and 'Abutment concrete'.

Anzac Bridge

周辺での地震記録

